

Beräknade och uppmätta rörelser hos stödkonstruktioner-Västlänken

E03 Kvarnberget

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HM Geokonsult AB



Agenda:

Kort information om projektet följt av redogörelse av beräknade och uppmätta rörelser i slitsmurar för södra tunneldelen.

E03 Kvarnberget, Totalentreprenad

Beställare: Trafikverket

Entreprenör: AGN Haga AB

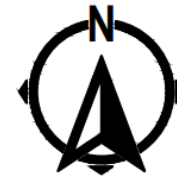
Konsulter: COWI, Ramböll och SGS

Slutbesiktning 15 maj 2024

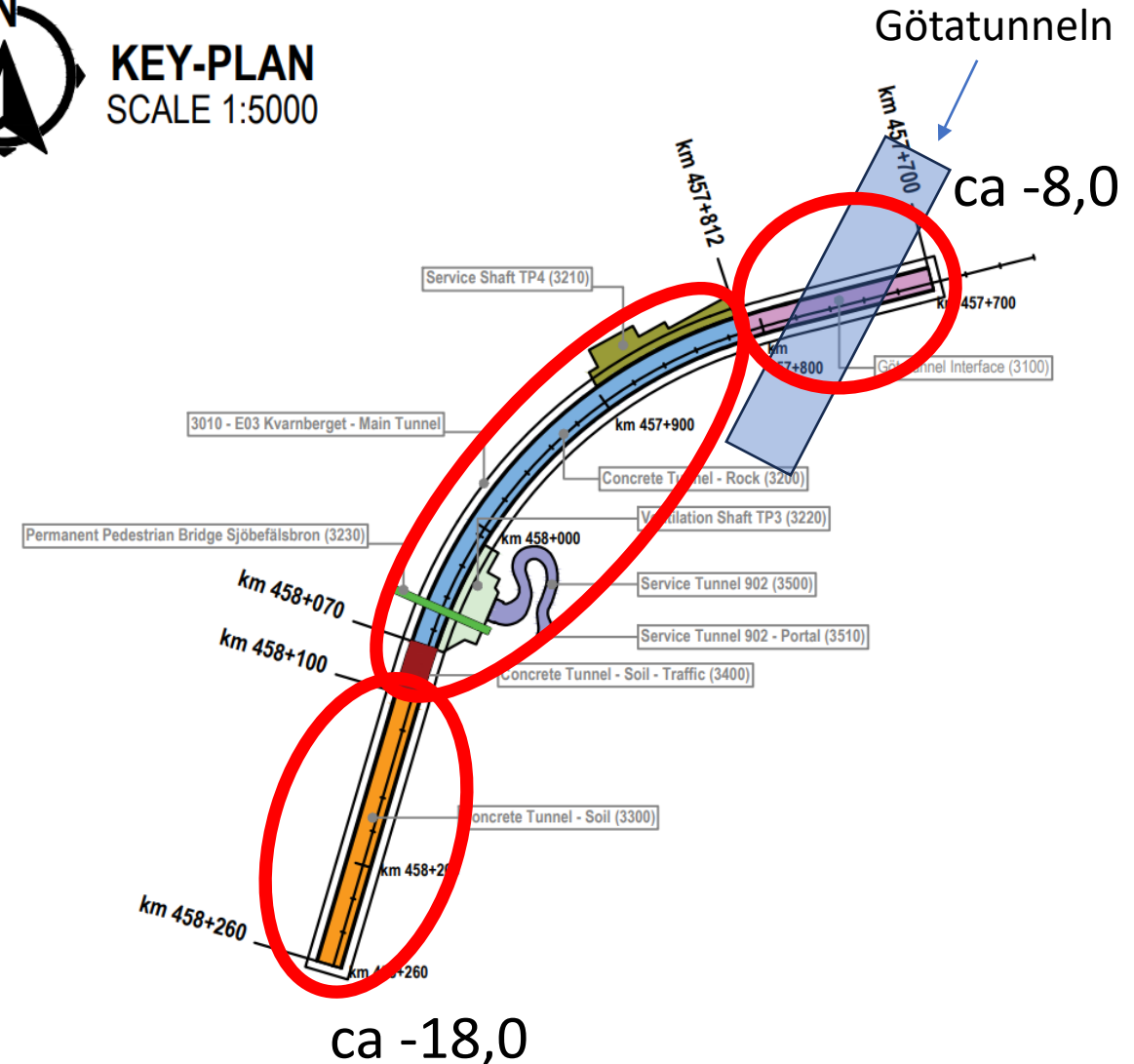
PÅLDAG 24

pågrundläggning

- Norra delen, Götatunneln, svävande spont, stämp och bergstag. Kc-i skivor passivzon. Stödmur. Anslutning E02. Schaktbotten ca -8,0
- Mitt delen, västra sidan spont slagen till berg. Stämp och bergstag (några jordstag). Förankrad kantbalk på berg. Östra sidan sågad eller sprängd bergvägg, förstärkningar. Schaktbotten ca -8 till -15.
- Södra delen, slitsmurar, top-down, kc-pelare i block. Anslutning E04. Schaktbotten ca -15 till -18.



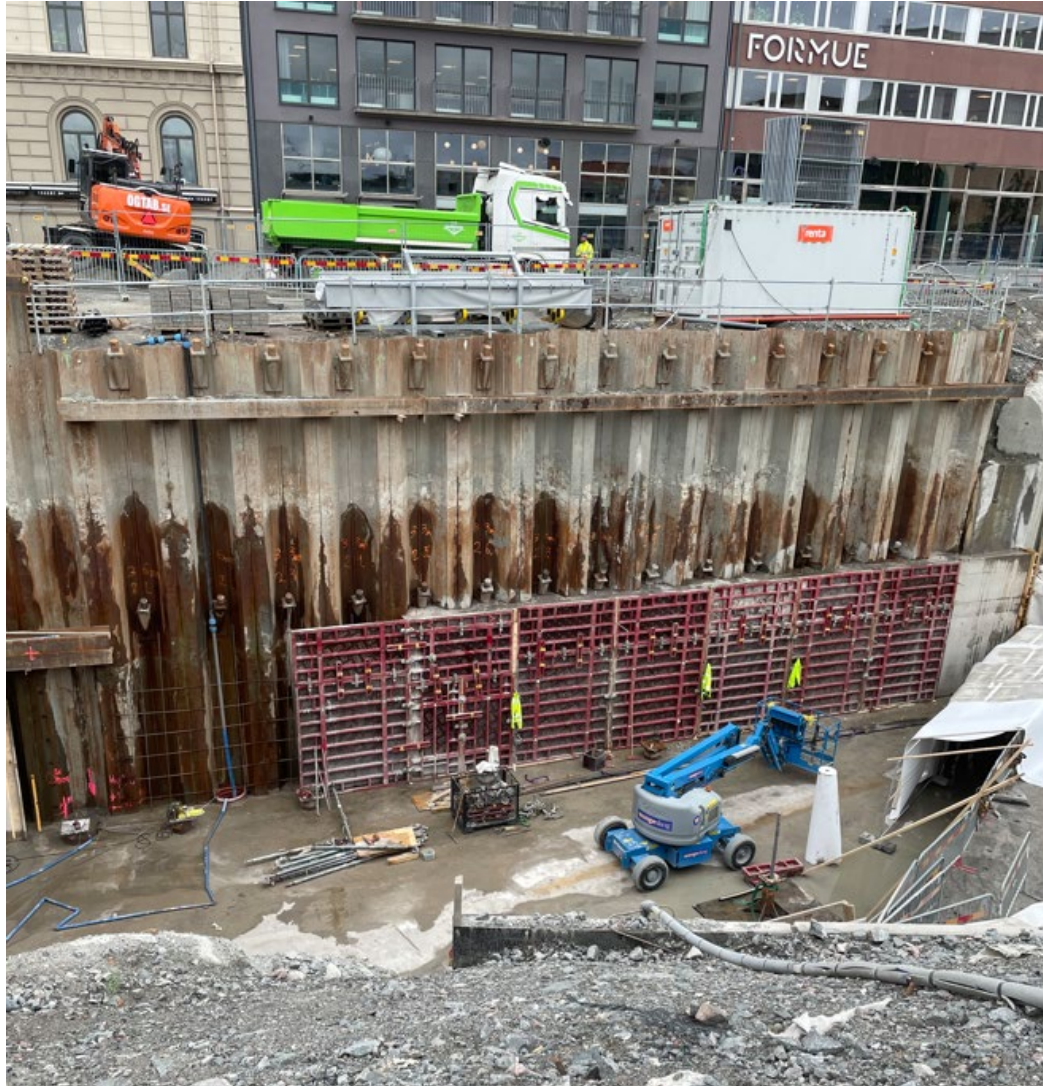
KEY-PLAN
SCALE 1:5000



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pålgrundläggning

Norra delen



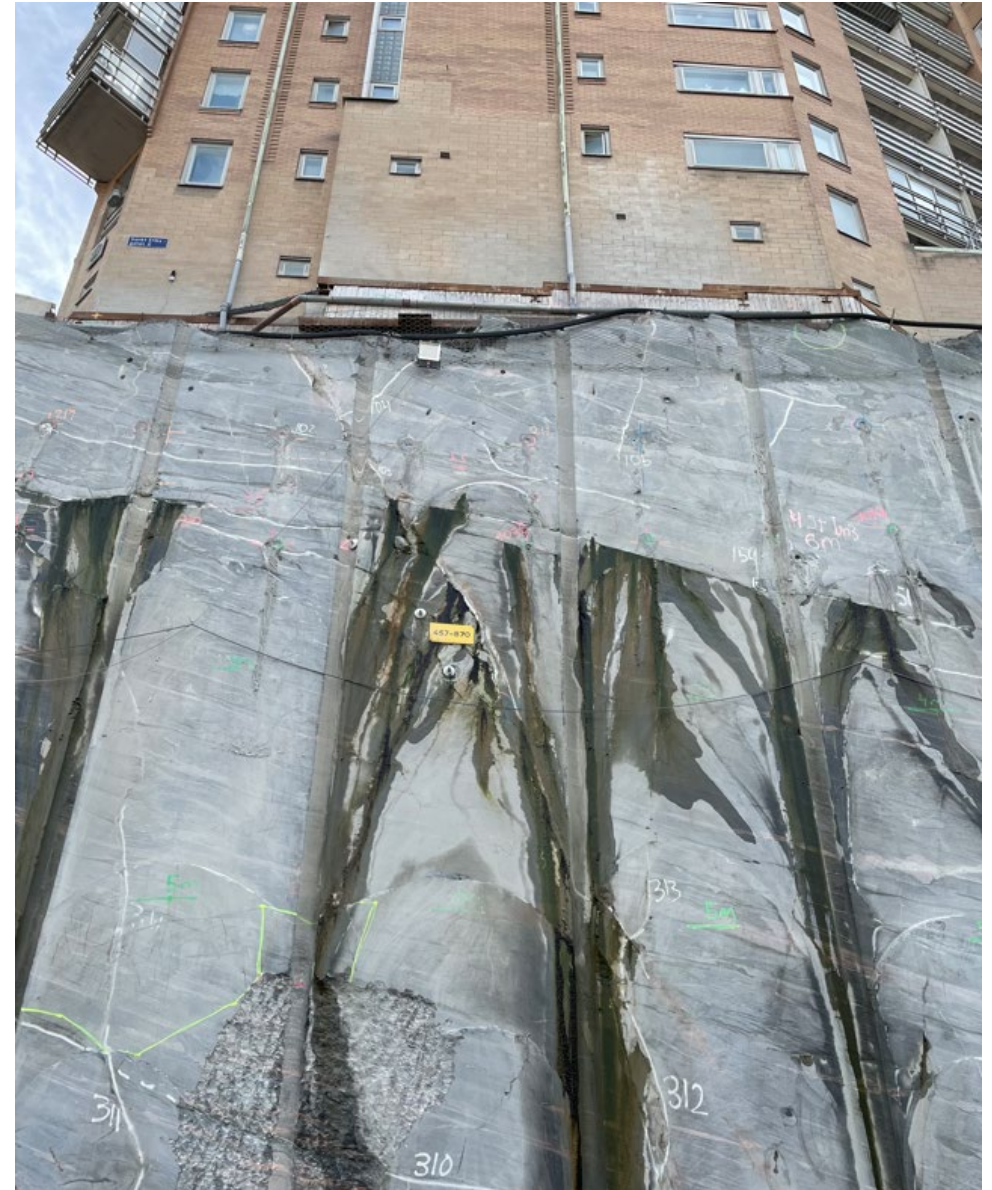
Norra delen E02



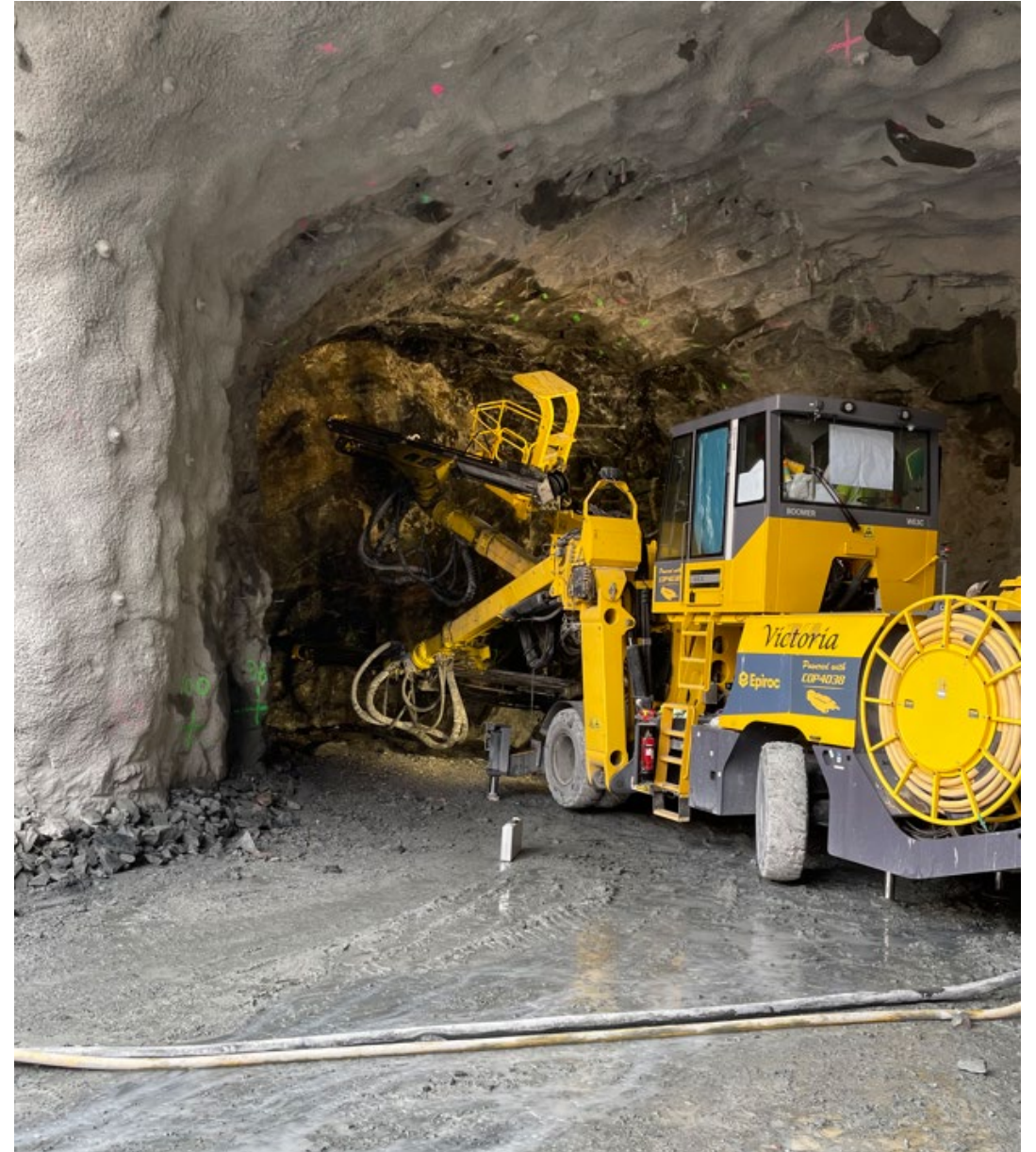
Mitten delen, västra sidan



Mitten delen, östra sidan



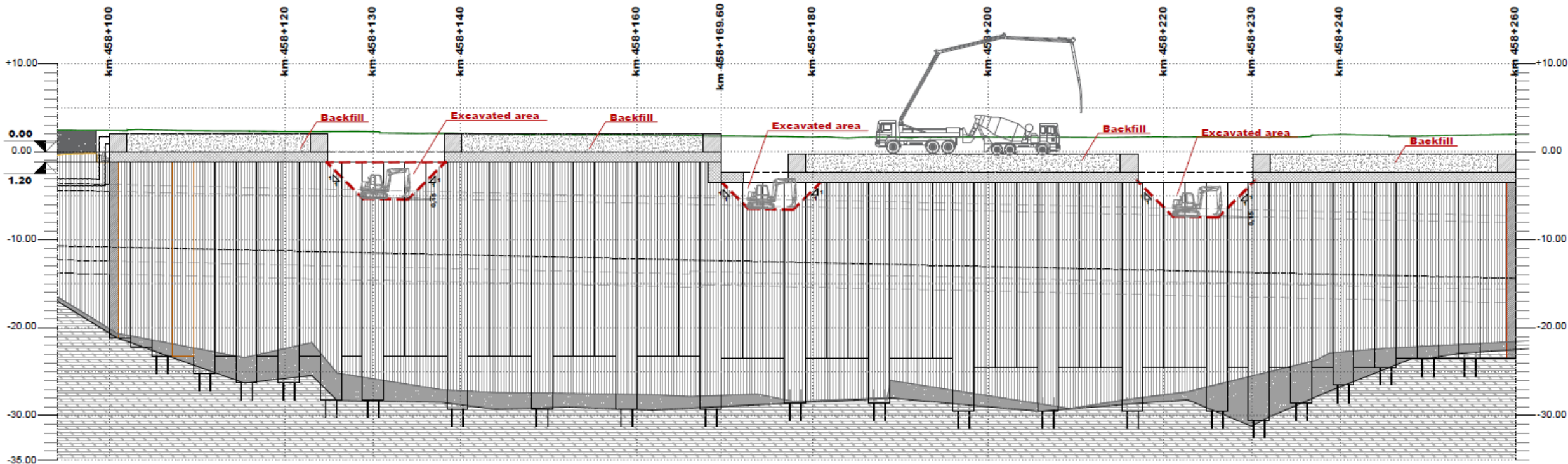
Mitten delen, Service tunnel



Mitten delen



LONGITUDINAL SECTION 1:500

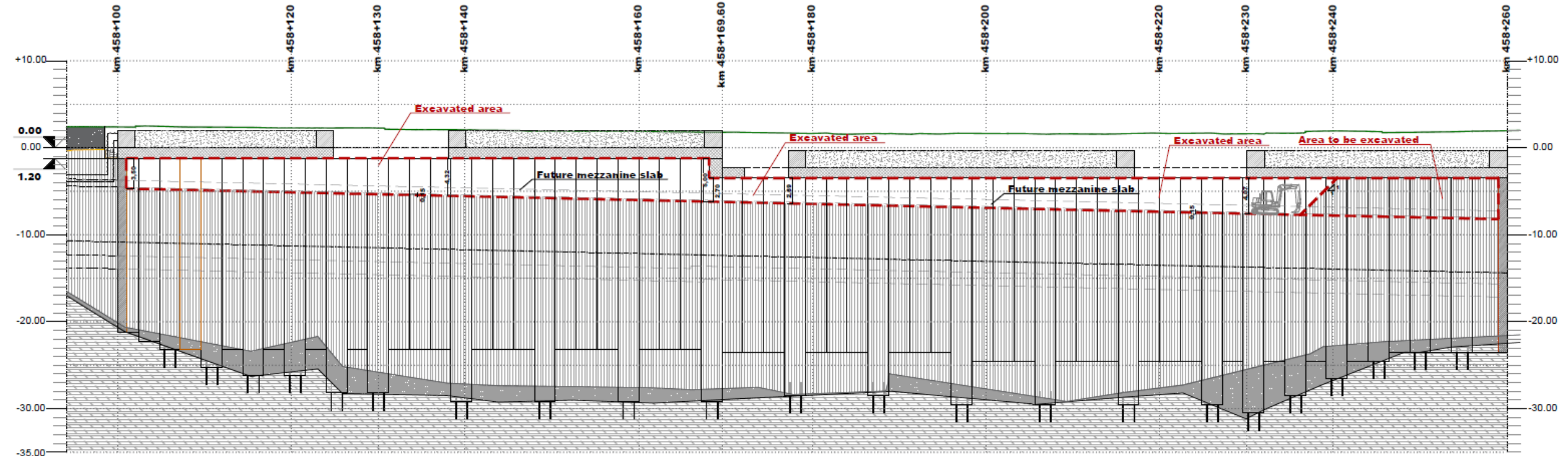


Phase 1: Excavation down to mezzanine slab level



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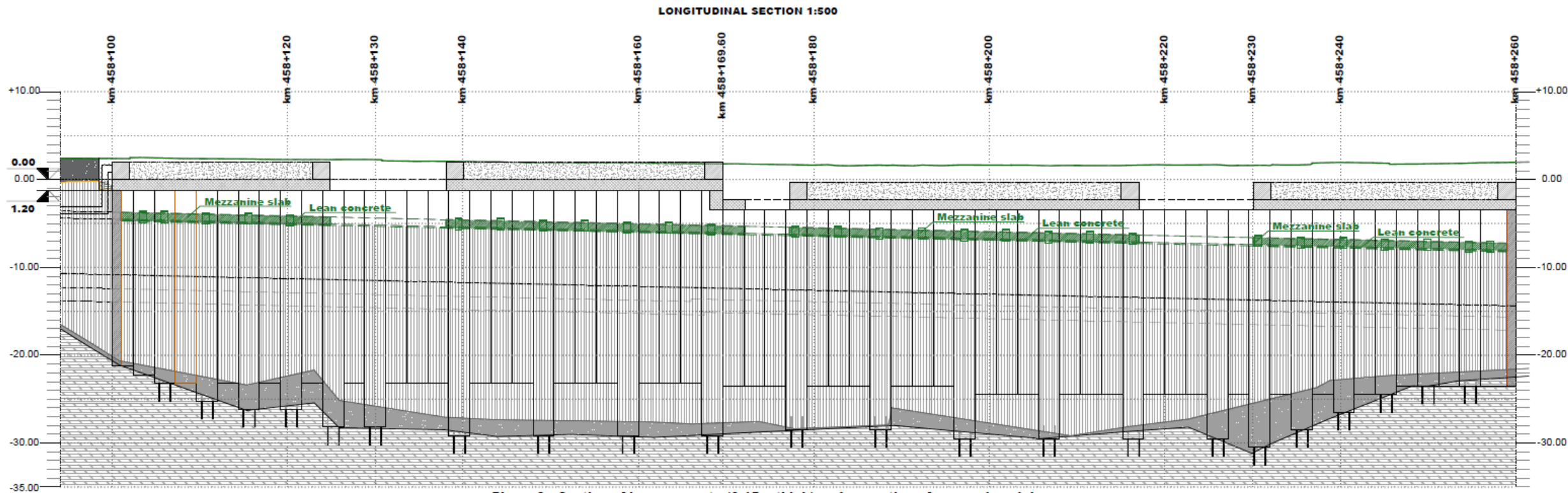
LONGITUDINAL SECTION 1:500



Phase 2: Excavation down to 0.15m below the bottom level of mezzanine slab



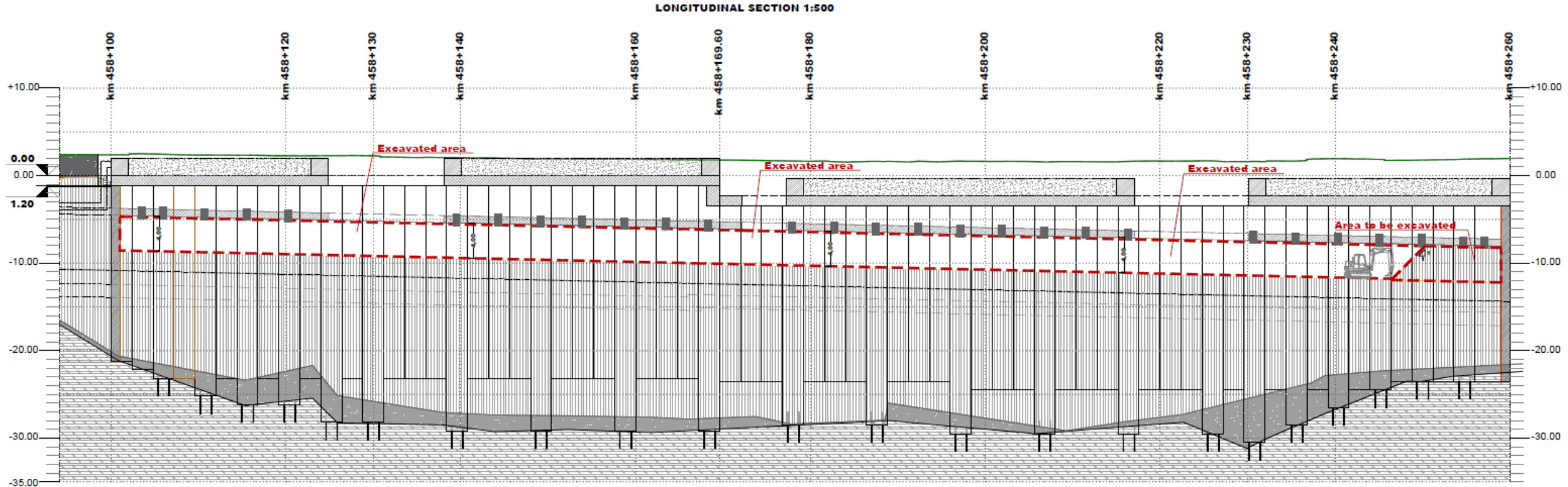
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Phase 3: Casting of lean concrete (0.15m thick) and execution of mezzanine slab



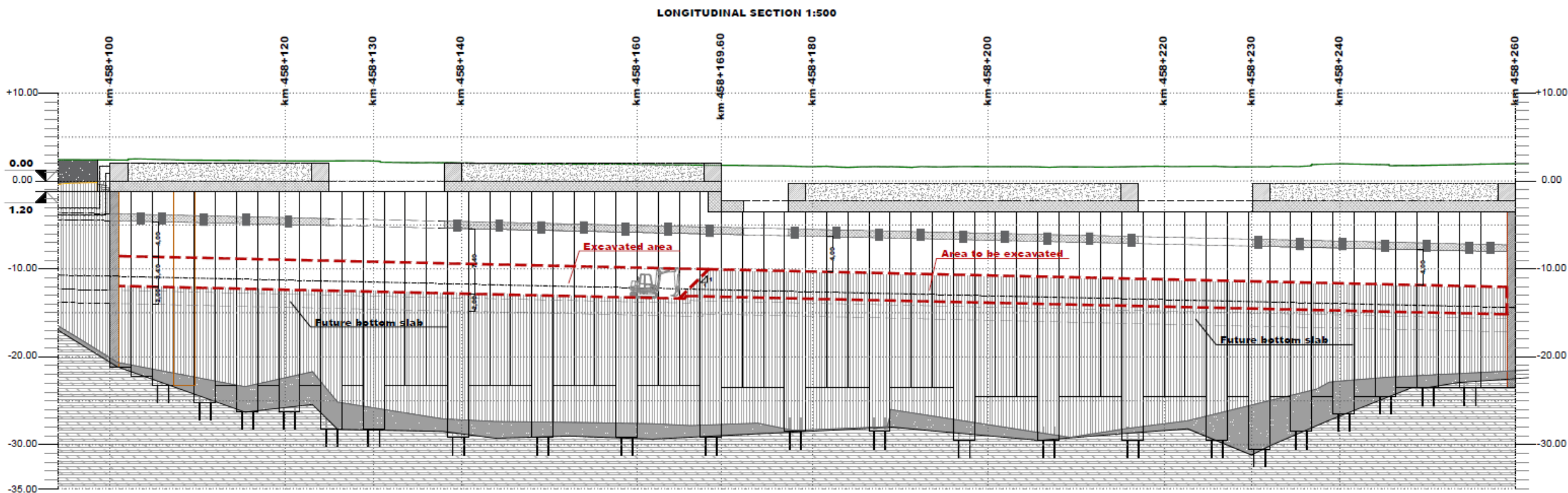
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Phase 4: Excavation down to 4.00m below the bottom level of mezzanine slab



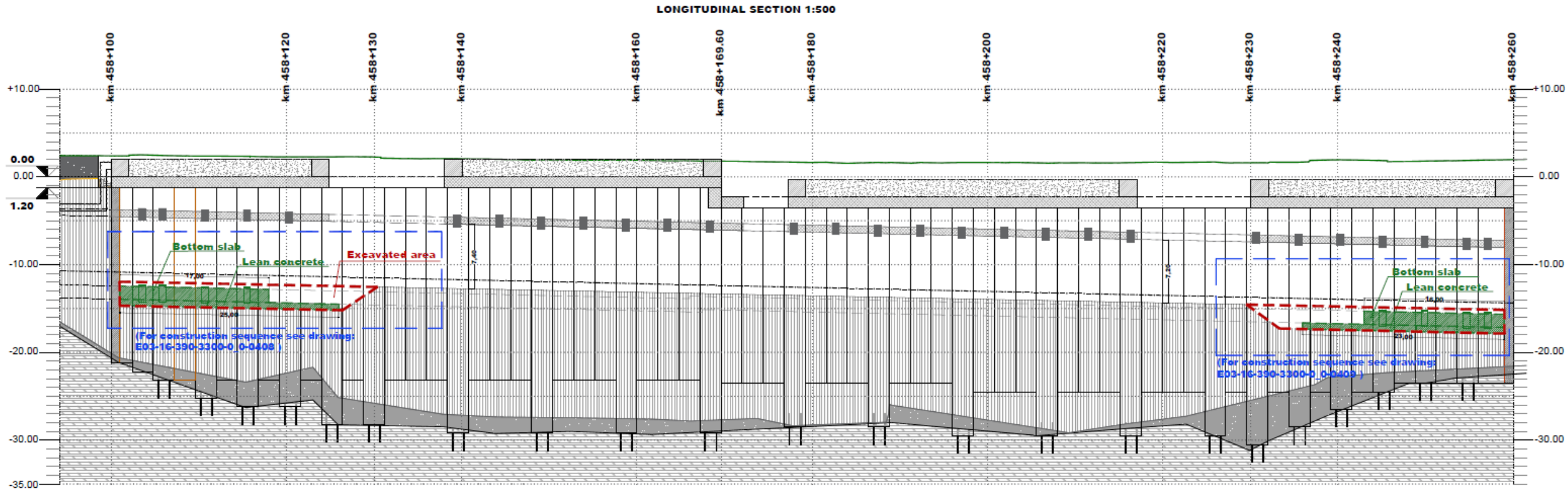
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Phase 5: Excavation of additional 3.4m (down to 2.00m over the bottom level of bottom slab)



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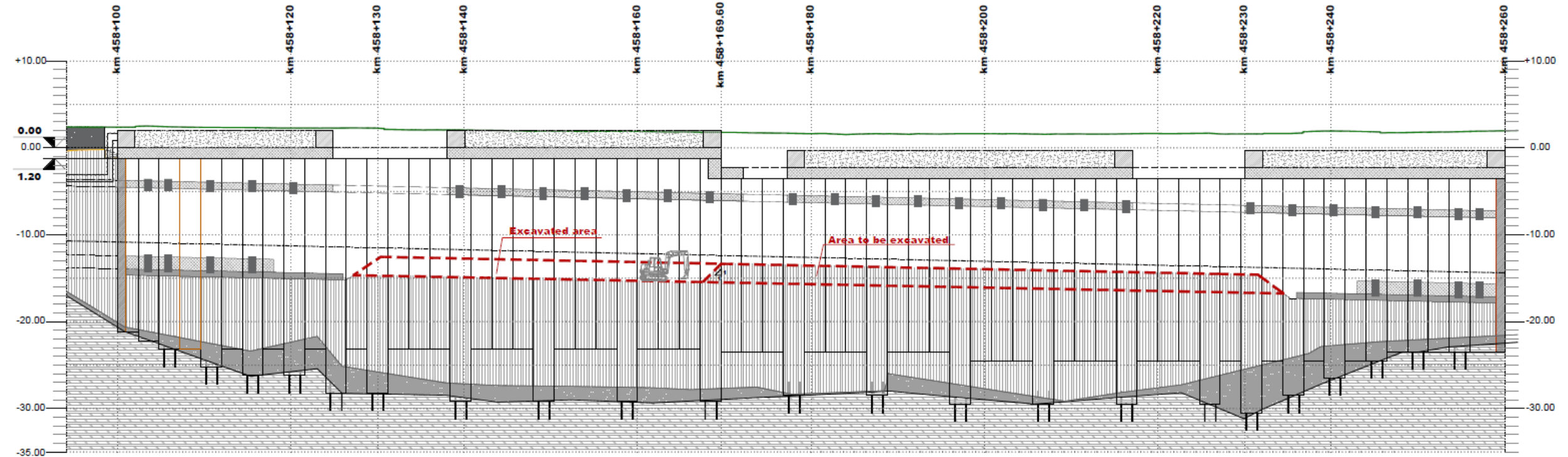


Phase 6: Local excavation, casting of lean concrete (0.70m thick) and partial execution of bottom slab at transition areas



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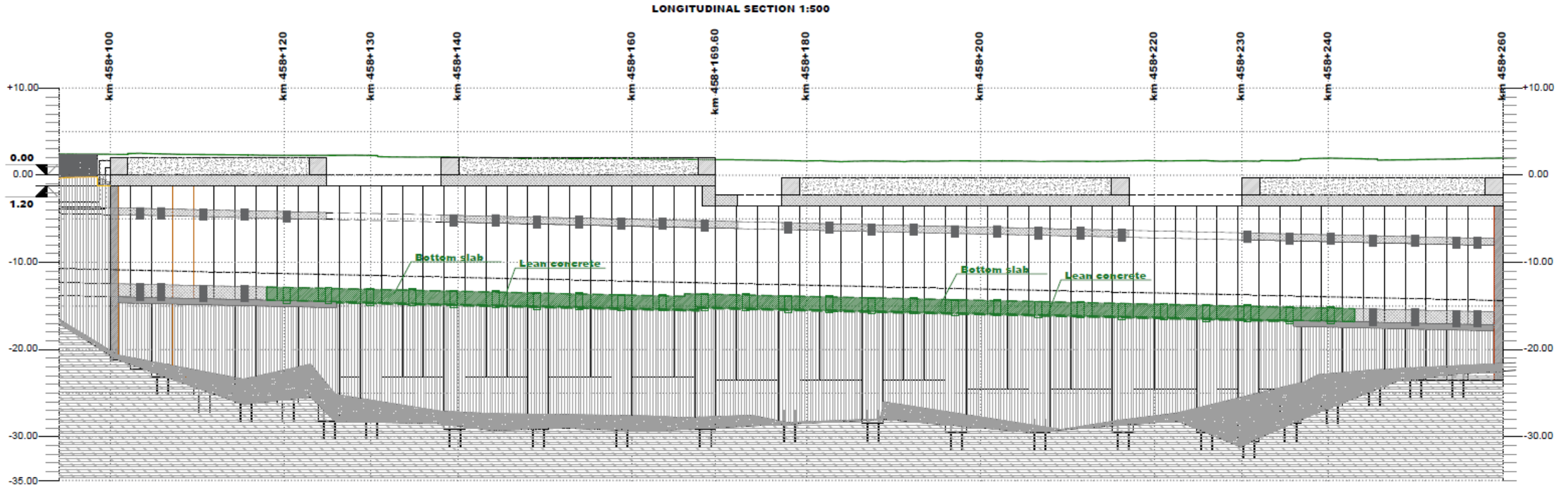
LONGITUDINAL SECTION 1:500



Phase 7: Excavation down to 0.15m below the bottom level of bottom slab



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Phase 8: Casting of lean concrete (0.15m thick) and full installation of bottom slab



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pålgrundläggning

Södra delen, slitsmurar, kc i block, top-down



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pålgrundläggning

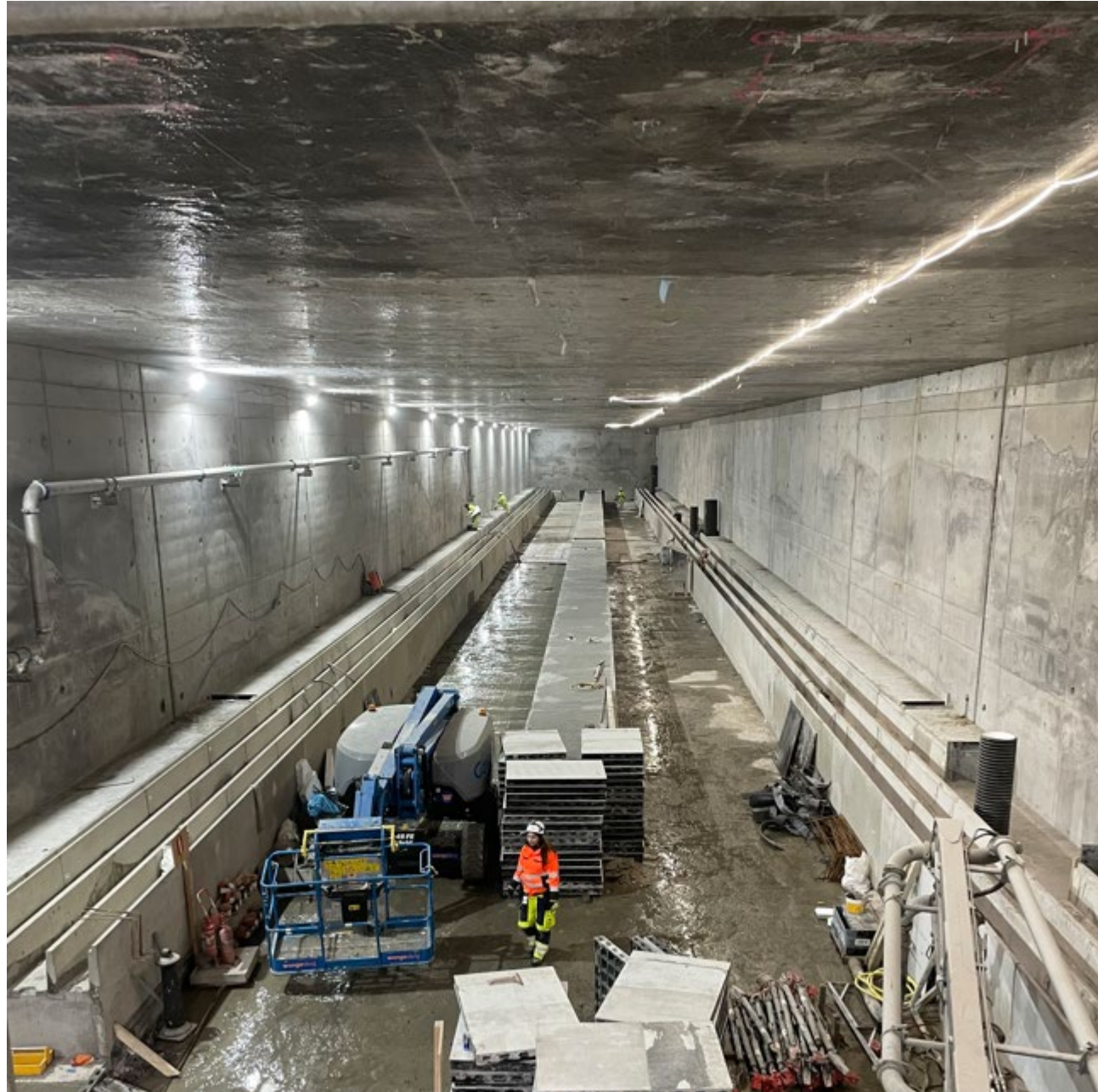
Södra delen



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pålgrundläggning

Södra delen



Kc-pelare i block, DSM A

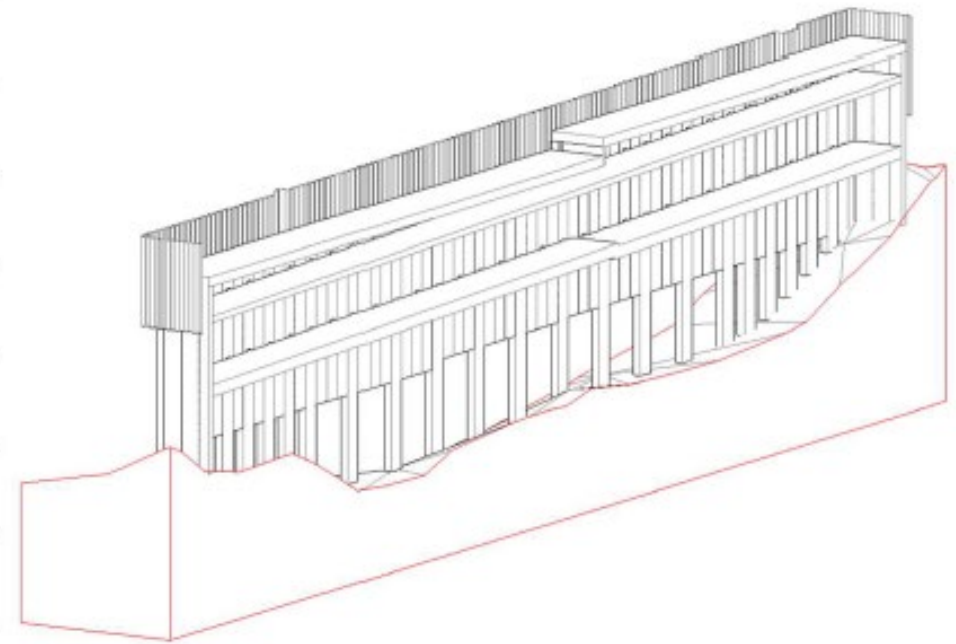
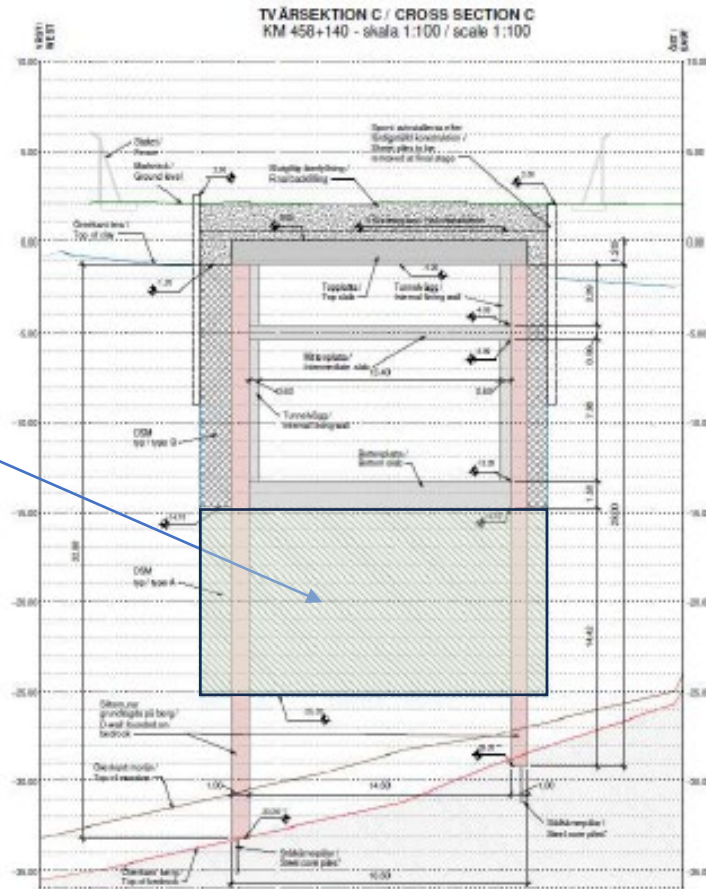


Figure 40 Typical section at Southern area (left) and 3D view (right)

Soil	Unit weight γ_{sat} [kN/m ³]	Cohesion c_u [kPa]	Elastic modulus E' [Mpa]	Poisson ratio ν [-]
DSM type B	16.5	100	52	0.3
DSM type A	16.5	250	130	0.3

Geometry

- column diameter is 800mm and spacing is 640mm
- two types of DSM are present: type A below excavation level and type B above excavation level

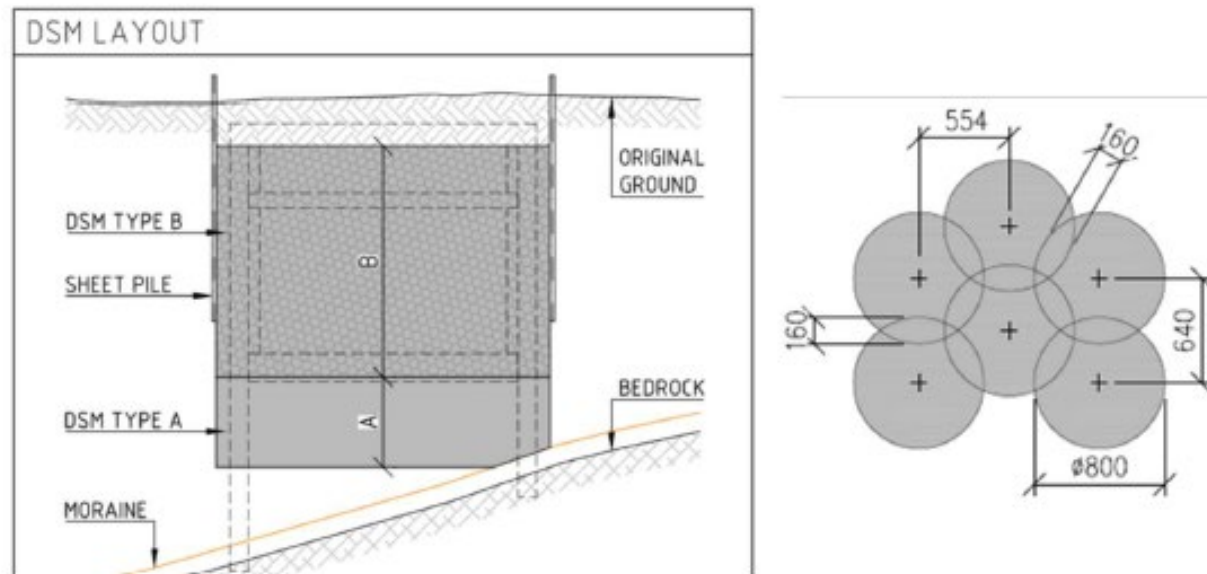


Figure 2 Main DSM geometry

DSM A, Kc-pelare i block tester

- n.89 of FKPS tests
- n.10 of FOPS tests
- n.9 of OPS tests
- n.38 of JB tests (done to verify bedrock level)
- coring with sample recovery and UCS tests (total of 14 tests from 4 boreholes)
- pressumeter tests (a total of 14 tests in 5 boreholes)
- down-hole and cross-hole tests (very limited)

Table 4 Summary of Mechanical Parameters

TEST TYPE	AVERAGE UNIT WEIGHT γ (kN/m ³)	AVERAGE SHEAR STRENGTH C_u (kPa)	AVERAGE STIFFNESS E' (Mpa)
FKPS	-	289-415	-
FOP / OPS	-	300	-
Lab tests on cores	17,1	555	-
Pressumeter	-	317-549	205-250
Seismic tests	-	-	345
JB soundings	-	-	-
DESIGN VALUES	16,5	250	130

Medelvärden!!



FKPS Shear strength [kPa] vs depth [m]

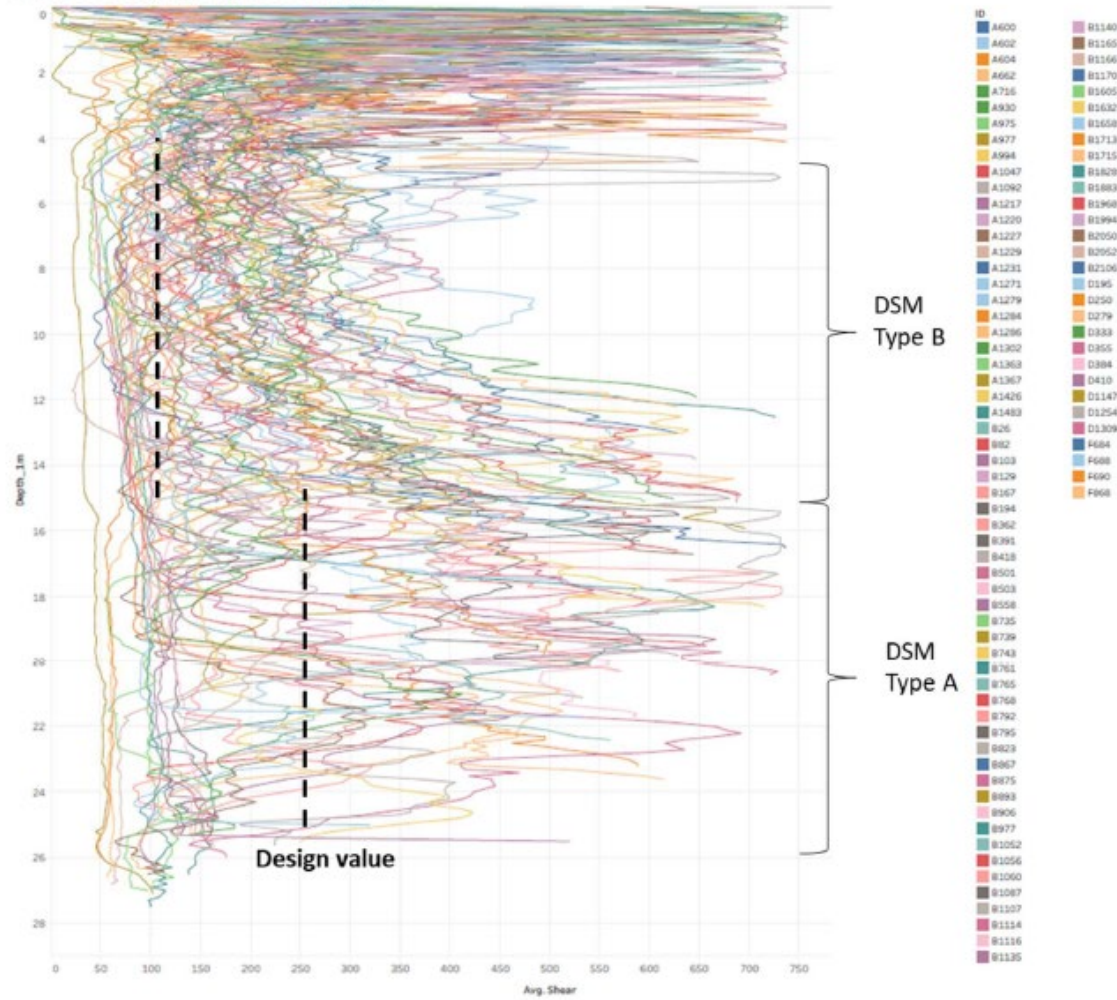


Figure 12 FKPS results

Pre-drilling force/FKPS

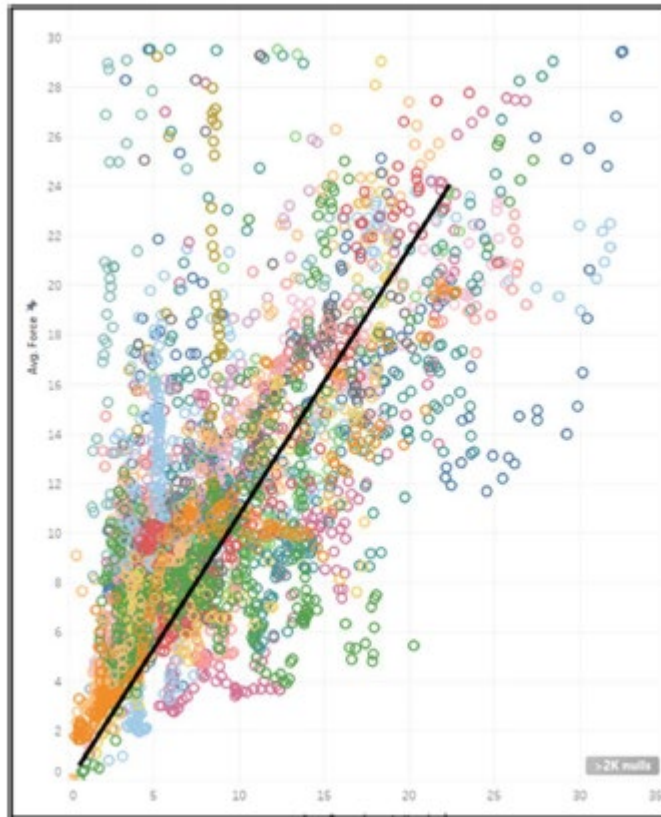
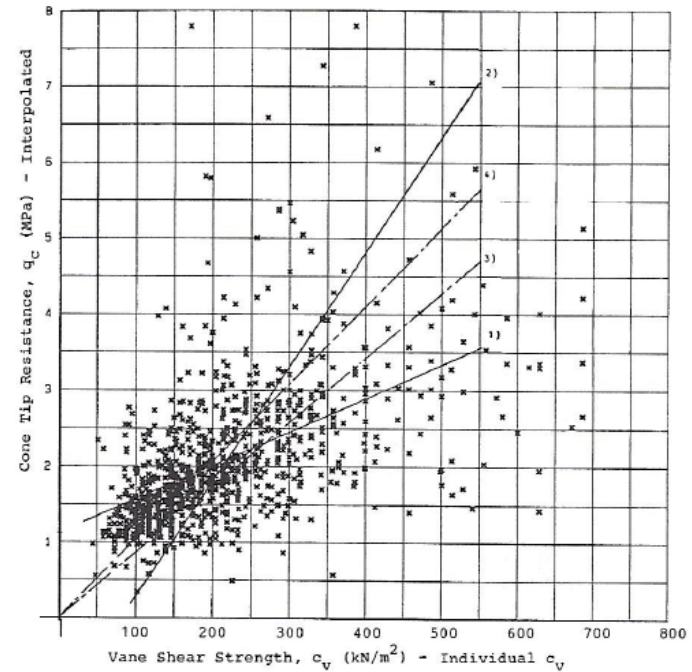


Figure 15 Correlation between pre-drilling force (x-axis) and wing force (y-axis)



- Note:
- 1) Simple linear regression, q_c as dependent variable ($q_c = 4.45 c_v + 1107.4$ (kN/m²))
 - 2) Simple linear regression, c_v as dependent variable ($c_v = 0.06734 q_c + 75.7$ (kN/m²) or $q_c = 14.85 c_v - 1124.1$ (kN/m²))
 - 3) Straight line regression through (0,0), q_c as dependent variable ($q_c = 8.51 c_v$ (kN/m²))
 - 4) Straight line regression through (0,0), c_v as dependent variable ($c_v = 0.09798 q_c$ (kN/m²) or $q_c = 10.20 c_v$ (kN/m²))

Fig. 12. Scatterplot q_c , c_v , individual c_v

Inklinometrar ingjutna i slitsmurar

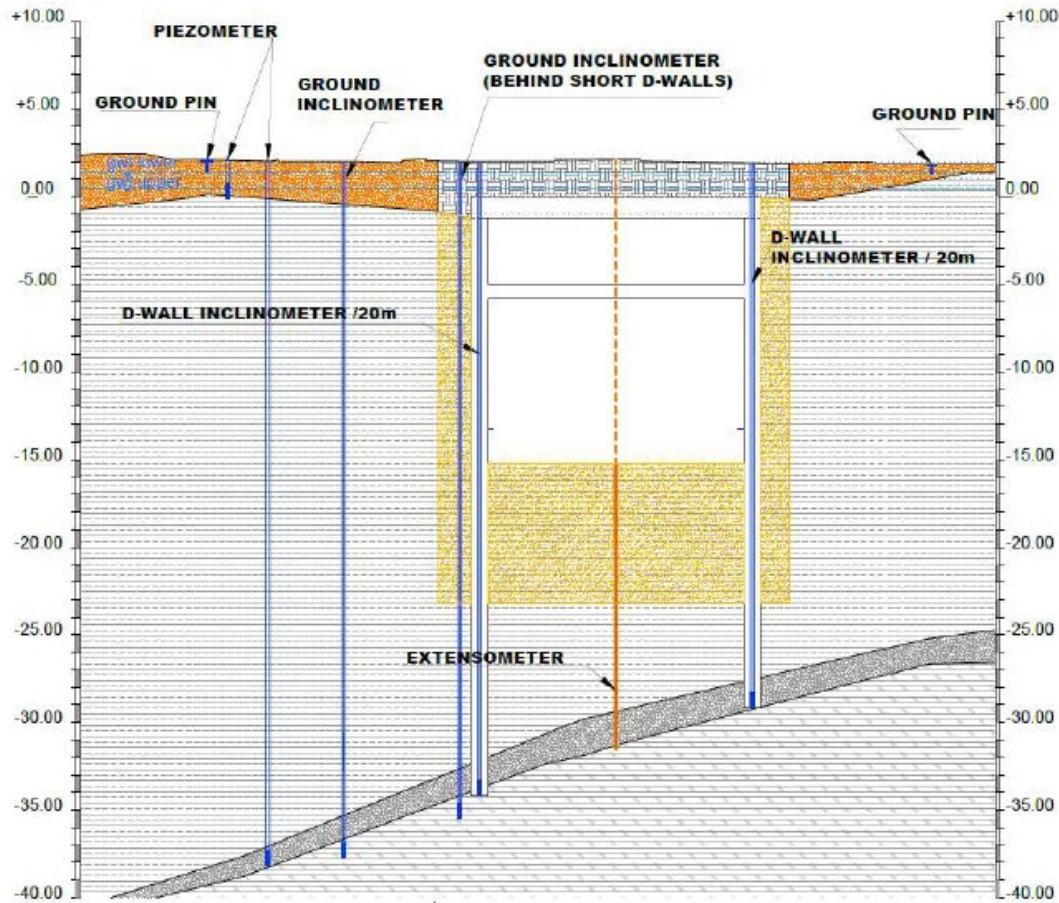
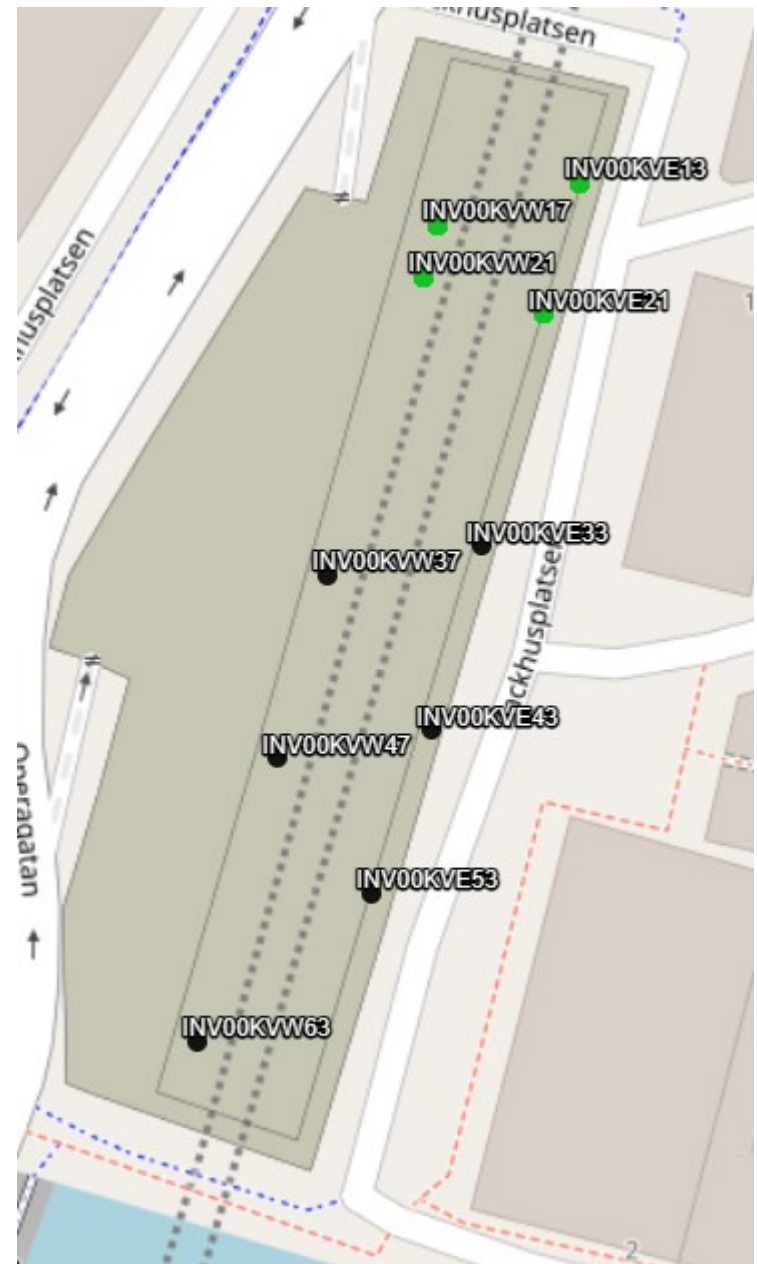
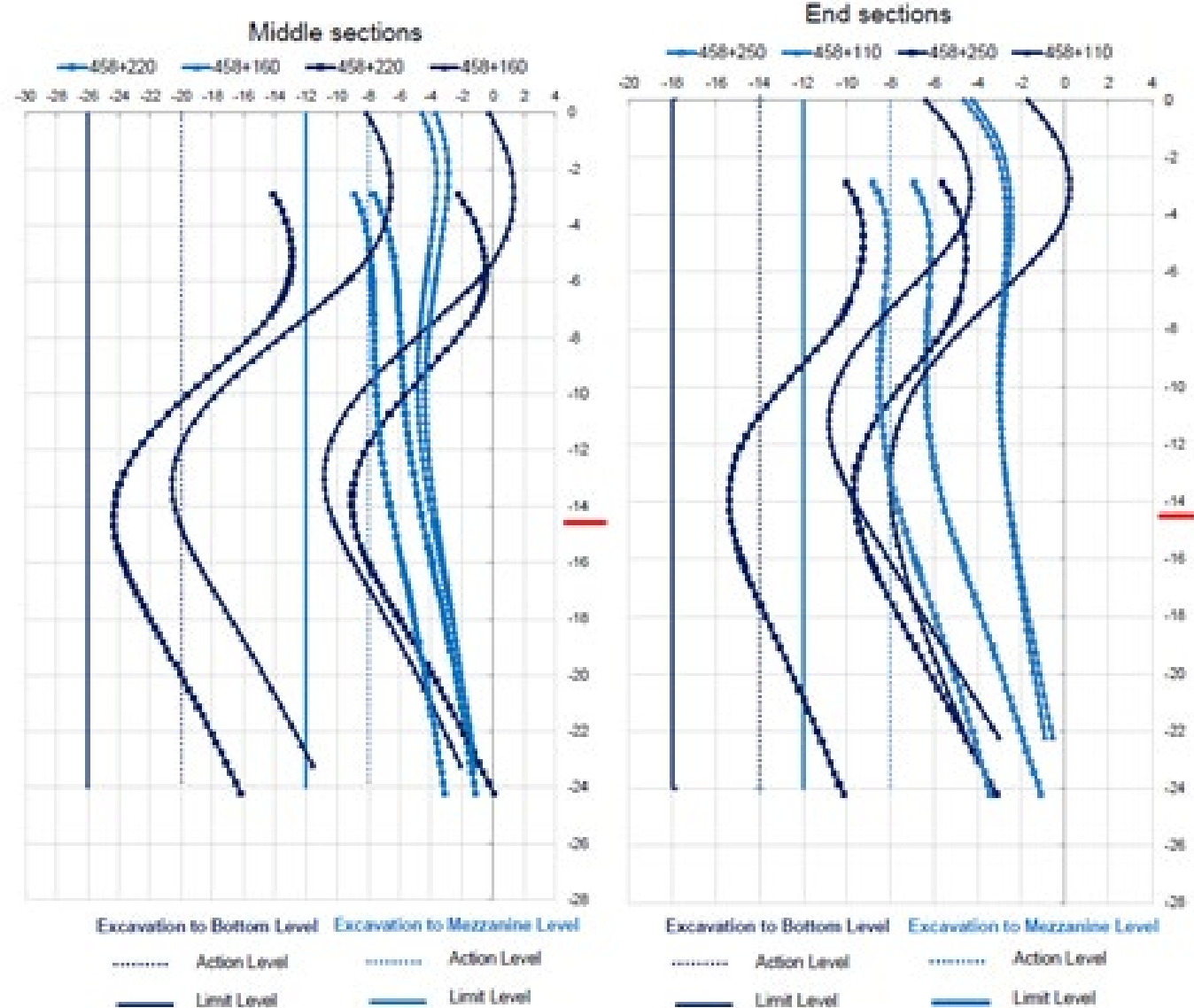


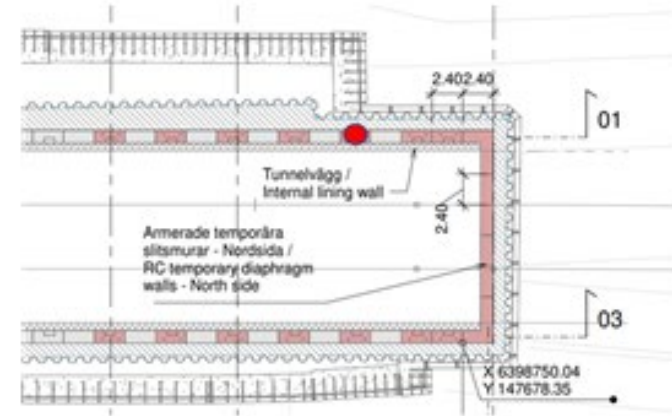
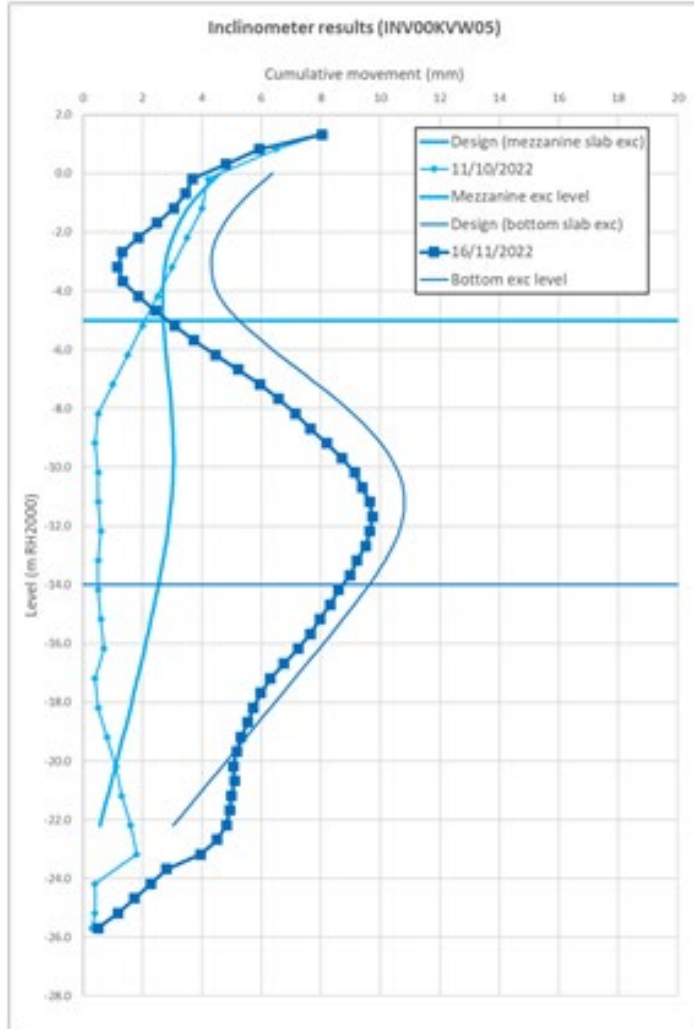
Figure 46 Typical monitoring section



Rörelse prognoser



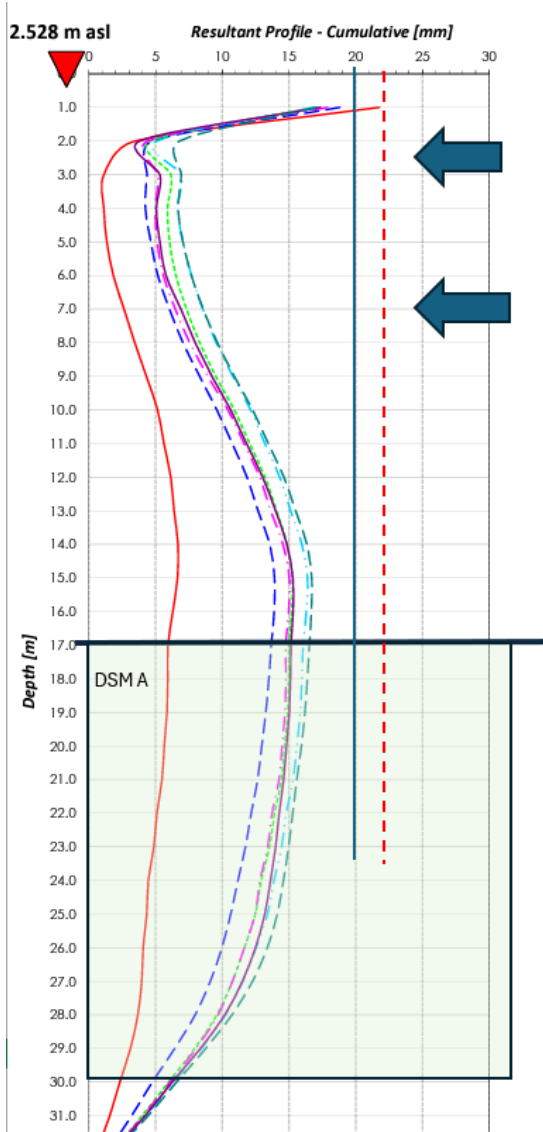
W05 panel



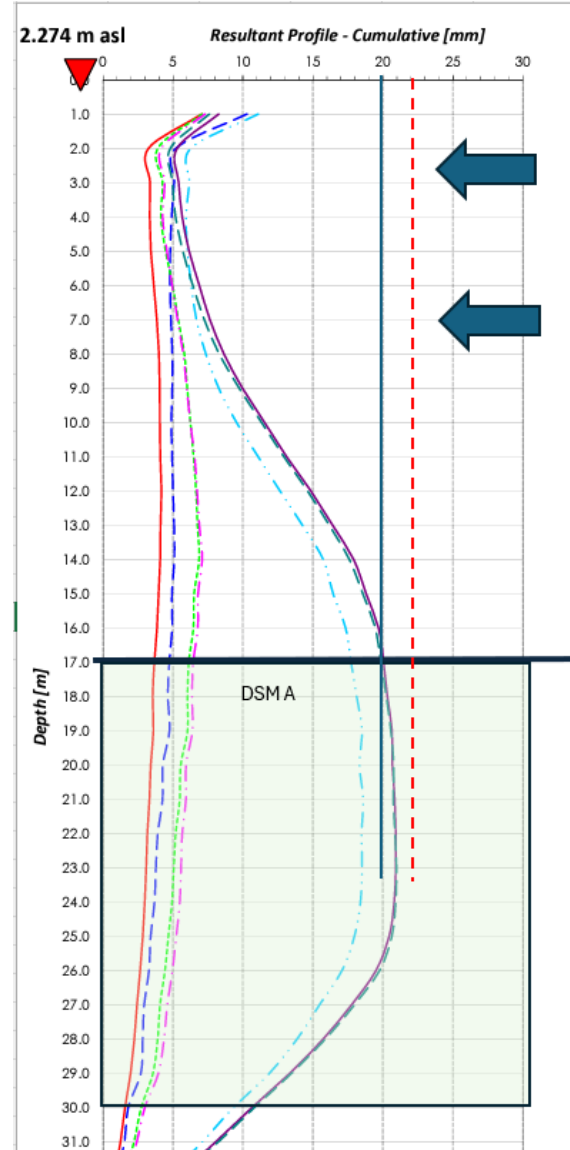
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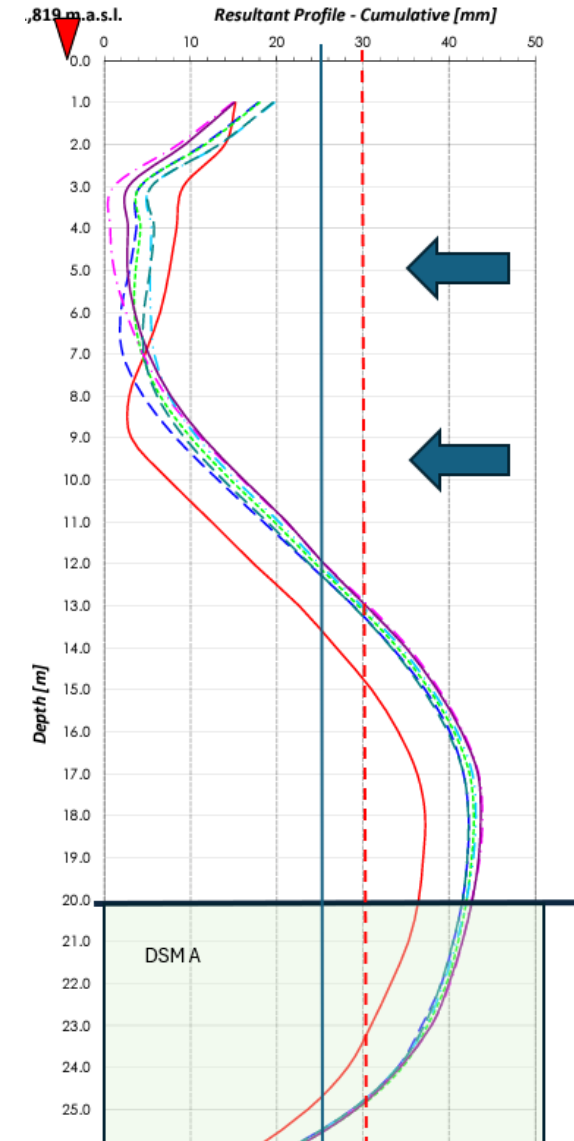
KVW17



KVW21



KVE63



Horisontalrörelser i schaktbotten

Inklinometer	Mätt (mm)	Prognos (mm)	Skillnad (%)
KVW05	13	11	118
KVE13	7	8	88
KVW17	16	20	80
KVW21	20	20	100
KVE21	11	20	55
KVE33	5	20	25
KVE43	3	25	12
KVE53	4	25	16
KVE63	42	25	168

E-Modul 130 MPa

Antagen bäddmodul 22500 kN/m³